Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System)\(^1\)

This standard is issued under the fixed designation D 2487; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (\(\epsilon\)) indicates an editorial change since the last revision or reapproval.

This standard has been approved for use by agencies of the Department of Defense.

1. Scope*

1.1 This practice describes a system for classifying mineral and organo-mineral soils for engineering purposes based on laboratory determination of particle-size characteristics, liquid limit, and plasticity index and shall be used when precise classification is required.

Note 1—Use of this standard will result in a single classification group symbol and group name except when a soil contains 5 to 12% fines or when the plot of the liquid limit and plasticity index values falls into the crosshatched area of the plasticity chart. In these two cases, a dual symbol is used, for example, GP-GM, CL-ML. When the laboratory test results indicate that the soil is close to another soil classification group, the borderline condition can be indicated with two symbols separated by a slash. The first symbol should be the one based on this standard, for example, CL/CH, GM/SM, SC/CL. Borderline symbols are particularly useful when the liquid limit value of clayey soils is close to 50. These soils can have expansive characteristics and the use of a borderline symbol (CL/CH, CH/CL) will alert the user of the assigned classifications of expansive potential.

1.2 The group symbol portion of this system is based on laboratory tests performed on the portion of a soil sample passing the 3-in. (75-mm) sieve (see Specification E11).

1.3 As a classification system, this standard is limited to naturally occurring soils.

Note 2—The group names and symbols used in this test method may be used as a descriptive system applied to such materials as shale, claystone, shells, crushed rock, etc. See Appendix X2.

1.4 This standard is for qualitative application only.

Note 3—When quantitative information is required for detailed designs of important structures, this test method must be supplemented by laboratory tests or other quantitative data to determine performance characteristics under expected field conditions.

1.5 This standard is the ASTM version of the Unified Soil Classification System. The basis for the classification scheme is the Airfield Classification System developed by A. Casagrande in the early 1940’s.\(^2\) It became known as the Unified Soil Classification System when several U.S. Government Agencies adopted a modified version of the Airfield System in 1952.

1.6 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

1.7 This practice offers a set of instructions for performing one or more specific operations. This document cannot replace education or experience and should be used in conjunction with professional judgment. Not all aspects of this practice may be applicable in all circumstances. This ASTM standard is not intended to represent or replace the standard of care by which the adequacy of a given professional service must be judged, nor should this document be applied without consideration of a project’s many unique aspects. The word “Standard” in the title of this document means only that the document has been approved through the ASTM consensus process.

2. Referenced Documents

2.1 ASTM Standards: \(^3\)

C 117 Test Method for Materials Finer than 75-µm (No. 200) Sieve in Mineral Aggregates by Washing

C 136 Test Method for Sieve Analysis of Fine and Coarse Aggregates

C 702 Practice for Reducing Samples of Aggregate to Testing Size

D 420 Guide to Site Characterization for Engineering Design and Construction Purposes

D 422 Test Method for Particle-Size Analysis of Soils

D 653 Terminology Relating to Soil, Rock, and Contained Fluids

\(^1\)This standard is under the jurisdiction of ASTM Committee D18 on Soil and Rock and is the direct responsibility of Subcommittee D18.07 on Identification and Classification of Soils.


\(^3\)For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For Annual Book of ASTM Standards volume information, refer to the standard’s Document Summary page on the ASTM website.
D 1140 Test Method for Amount of Material in Soils Finer Than the No. 200 (75-µm) Sieve
D 2216 Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass
D 2217 Practice for Wet Preparation of Soil Samples for Particle-Size Analysis and Determination of Soil Constants
D 2488 Practice for Description and Identification of Soils (Visual-Manual Procedure)
D 3740 Practice for Minimum Requirements for Agencies Engaged in the Testing and/or Inspection of Soil and Rock as Used in Engineering Design and Construction
D 4083 Practice for Description of Frozen Soils (Visual-Manual Procedure)
D 4318 Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
D 4427 Classification of Peat Samples by Laboratory Testing
D 6913 Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis
E 11 Specification for Wire Cloth and Sieves for Testing Purposes

3. Terminology

3.1 Definitions—Except as listed below, all definitions are in accordance with Terminology D 653.

Note 4—For particles retained on a 3-in. (75-mm) U.S. standard sieve, the following definitions are suggested:

Cobbles—particles of rock that will pass a 12-in. (300-mm) square opening and be retained on a 3-in. (75-mm) U.S. standard sieve, and

Boulders—particles of rock that will not pass a 12-in. (300-mm) square opening.

3.1.1 clay—soil passing a No. 200 (75-µm) U.S. standard sieve that can be made to exhibit plasticity (putty-like properties) within a range of water contents and that exhibits considerable strength when air dry. For classification, a clay is a fine-grained soil, or the fine-grained portion of a soil, with a plasticity index equal to or greater than 4, and the plot of plasticity index versus liquid limit falls on or above the “A” line.

3.1.2 gravel—particles of rock that will pass a 3-in. (75-mm) sieve and be retained on a No. 4 (4.75-mm) U.S. standard sieve with the following subdivisions:

Coarse—passes 3-in. (75-mm) sieve and retained on ¾-in. (19-mm) sieve, and

Fine—passes ¾-in. (19-mm) sieve and retained on No. 4 (4.75-mm) sieve.

3.1.3 organic clay—a clay with sufficient organic content to influence the soil properties. For classification, an organic clay is a soil that would be classified as a clay except that its liquid limit value after oven drying is less than 75 % of its liquid limit value before oven drying.

3.1.4 organic silt—a silt with sufficient organic content to influence the soil properties. For classification, an organic silt is a soil that would be classified as a silt except that its liquid limit value after oven drying is less than 75 % of its liquid limit value before oven drying.

3.1.5 peat—a soil composed of vegetable tissue in various stages of decomposition usually with an organic odor, a dark-brown to black color, a spongy consistency, and a texture ranging from fibrous to amorphous.

3.1.6 sand—particles of rock that will pass a No. 4 (4.75-mm) sieve and be retained on a No. 200 (75-µm) U.S. standard sieve with the following subdivisions:

Coarse—passes No. 4 (4.75-mm) sieve and retained on No. 10 (2.00-mm) sieve,

Medium—passes No. 10 (2.00-mm) sieve and retained on No. 40 (425-µm) sieve, and

Fine—passes No. 40 (425-µm) sieve and retained on No. 200 (75-µm) sieve.

3.1.7 silt—soil passing a No. 200 (75-µm) U.S. standard sieve that is nonplastic or very slightly plastic and that exhibits little or no strength when air dry. For classification, a silt is a fine-grained soil, or the fine-grained portion of a soil, with a plasticity index less than 4 or if the plot of plasticity index versus liquid limit falls below the “A” line.

3.2 Definitions of Terms Specific to This Standard:

3.2.1 coefficient of curvature, Cc—the ratio \( (D_{30})^2 / (D_{10} \times D_{60}) \), where \( D_{40}, D_{30}, \) and \( D_{10} \) are the particle sizes corresponding to 60, 30, and 10 % finer on the cumulative particle-size distribution curve, respectively.

3.2.2 coefficient of uniformity, Cu—the ratio \( D_{60}/D_{10} \), where \( D_{60} \) and \( D_{10} \) are the particle diameters corresponding to 60 and 10 % finer on the cumulative particle-size distribution curve, respectively.

4. Summary

4.1 As illustrated in Table 1, this classification system identifies three major soil divisions: coarse-grained soils, fine-grained soils, and highly organic soils. These three divisions are further subdivided into a total of 15 basic soil groups.

### Table 1 Soil Classification Chart

<table>
<thead>
<tr>
<th>Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests(^4)</th>
<th>Soil Classification</th>
</tr>
</thead>
<tbody>
<tr>
<td>Group Symbol</td>
<td>Group Name(^4)</td>
</tr>
<tr>
<td>COARSE-GRAINED SOILS</td>
<td>Gravels</td>
</tr>
<tr>
<td>More than 50 % retained on No. 200 sieve</td>
<td>More than 50 % of coarse fraction retained on No. 4 sieve</td>
</tr>
<tr>
<td>Gravels with Fines</td>
<td>Fines classify as ML or MH</td>
</tr>
</tbody>
</table>
5. Significance and Use

5.1 This standard classifies soils from any geographic location into categories representing the results of prescribed laboratory tests to determine the particle-size characteristics, the liquid limit, and the plasticity index.

5.2 The assigning of a group name and symbol(s) along with the descriptive information required in Practice D 2488 can be used to describe a soil to aid in the evaluation of its significant properties for engineering use.

5.3 The various groupings of this classification system have been devised to correlate in a general way with the engineering behavior of soils. This standard provides a useful first step in any field or laboratory investigation for geotechnical engineering purposes.

5.4 This standard may also be used as an aid in training personnel in the use of Practice D 2488.

5.5 This standard may be used in combination with Practice D 4083 when working with frozen soils.

NOTE 5—Notwithstanding the statements on precision and bias contained in this standard: The precision of this test method is dependent on the competence of the personnel performing it and the suitability of the equipment and facilities used. Agencies that meet the criteria of Practice D 3740 are generally considered capable of competent and objective testing. Users of this test method are cautioned that compliance with Practice D 3740 does not in itself assure reliable testing. Reliable testing...
FIG. 1 Flow Chart for Classifying Fine-Grained Soil (50 % or More Passes No. 200 Sieve)

FIG. 2 Flow Chart for Classifying Organic Fine-Grained Soil (50 % or More Passes No. 200 Sieve)
depends on several factors; Practice D 3740 provides a means for evaluating some of those factors.

6. Apparatus

6.1 In addition to the apparatus that may be required for obtaining and preparing the samples and conducting the prescribed laboratory tests, a plasticity chart, similar to Fig. 4, and a cumulative particle-size distribution curve, similar to Fig. 5, are required.

Note 6—The “U” line shown on Fig. 4 has been empirically determined to be the approximate “upper limit” for natural soils. It is a good check against erroneous data, and any test results that plot above or to the left of it should be verified.

7. Sampling

7.1 Samples shall be obtained and identified in accordance with a method or methods, recommended in Guide D 420 or by other accepted procedures.

7.2 Test Methods D 6913 provides guidance on selecting size of specimen. Two test methods are provided in this standard. The methods differ in the significant digits recorded and the size of the specimen (mass) required. The method to be used may be specified by the requesting authority; otherwise Method A shall be performed. Whenever possible, the field samples should have weights two to four times larger than shown.

7.3 If the field sample or test specimen is smaller than the minimum recommended amount, the report shall include an appropriate remark.

8. Classification of Peat

8.1 A sample composed primarily of vegetable tissue in various stages of decomposition and has a fibrous to amorphous texture, a dark-brown to black color, and an organic odor should be designated as a highly organic soil and shall be classified as peat, PT, and not subjected to the classification procedures described hereafter.

8.2 If desired, classification of type of peat can be performed in accordance with Classification D 4427.

9. Preparation for Classification

9.1 Before a soil can be classified according to this standard, generally the particle-size distribution of the minus 3-in. (75-mm) material and the plasticity characteristics of the minus No. 40 (425-µm) sieve material must be determined. See 9.8 for the specific required tests.

9.2 The preparation of the soil specimen(s) and the testing for particle-size distribution and liquid limit and plasticity index shall be in accordance with accepted standard procedures. Two procedures for preparation of the soil specimens for testing for soil classification purposes are given in Appendices X3 and X4. Appendix X3 describes the wet preparation method...
For classification of fine-grained soils and fine-grained fraction of coarse-grained soils.

Equation of "A"-line
Horizontal at PI=4 to LL=25.5, then PI = 0.73 (LL-20)

Equation of "U"-line
Vertical at LL=16 to PI=7, then PI = 0.9 (LL-8)

FIG. 4 Plasticity Chart

FIG. 5 Cumulative Particle-Size Plot
and is the preferred method for cohesive soils that have never dried out and for organic soils.

9.3 When reporting soil classifications determined by this standard, the preparation and test procedures used shall be reported or referenced.

9.4 Although the test procedure used in determining the particle-size distribution or other considerations may require a hydrometer analysis of the material, a hydrometer analysis is not necessary for soil classification.

9.5 The percentage (by dry weight) of any plus 3-in. (75-mm) material must be determined and reported as auxiliary information.

9.6 The maximum particle size shall be determined (measured or estimated) and reported as auxiliary information.

9.7 When the cumulative particle-size distribution is required, a set of sieves shall be used which include the following sizes (with the largest size commensurate with the maximum particle size) with other sieve sizes as needed or required to define the particle-size distribution:

3-in. (75-mm)  
1/4-in. (19.0-mm)  
No. 4 (4.75-mm)  
No. 10 (2.00-mm)  
No. 40 (425-µm)  
No. 200 (75-µm)

9.8 The tests required to be performed in preparation for classification are as follows:

9.8.1 For soils estimated to contain less than 5 % fines, a plot of the cumulative particle-size distribution curve of the fraction coarser than the No. 200 (75-µm) sieve is required. A semi-log plot of percent passing versus particle-size or sieve size/sieve number is plotted as shown in Fig. 5.

9.8.2 For soils estimated to contain 5 to 15 % fines, a cumulative particle-size distribution curve, as described in 9.8.1, is required, and the liquid limit and plasticity index are required.

9.8.2.1 If sufficient material is not available to determine the liquid limit and plasticity index, the fines should be estimated to be either silty or clayey using the procedures described in Practice D 2488 and so noted in the report.

9.8.3 For soils estimated to contain 15 % or more fines, a determination of the percent fines, percent sand, and percent gravel is required, and the liquid limit and plasticity index are required. For soils estimated to contain 90 % fines or more, the percent fines, percent sand, and percent gravel may be estimated using the procedures described in Practice D 2488 and so noted in the report.

10. Preliminary Classification Procedure

10.1 Class the soil as fine-grained if 50 % or more by dry weight of the test specimen passes the No. 200 (75-µm) sieve and follow Section 3.1.2.

10.2 Class the soil as coarse-grained if more than 50 % by dry weight of the test specimen is retained on the No. 200 (75-µm) sieve and follow Section 12.

11. Procedure for Classification of Fine-Grained Soils

11.1 The soil is an inorganic clay if the position of the plasticity index versus liquid limit plot, Fig. 4, falls on or above the “A” line, the plasticity index is greater than 4, and the presence of organic matter does not influence the liquid limit as determined in 11.3.2.

NOTE 7—The plasticity index and liquid limit are determined on the minus No. 40 (425 µm) sieve material.

11.1.1 Classify the soil as a lean clay, CL, if the liquid limit is less than 50. See area identified as CL on Fig. 4.

11.1.2 Classify the soil as a fat clay, CH, if the liquid limit is 50 or greater. See area identified as CH on Fig. 4.

NOTE 8—In cases where the liquid limit exceeds 110 or the plasticity index exceeds 60, the plasticity chart may be expanded by maintaining the same scale on both axes and extending the “A” line at the indicated slope.

11.1.3 Classify the soil as a silty clay, CL-ML, if the position of the plasticity index versus liquid limit plot falls on or above the “A” line and the plasticity index is in the range of 4 to 7. See area identified as CL-ML on Fig. 4.

11.2 The soil is an inorganic silt if the position of the plasticity index versus liquid limit plot, Fig. 4, falls below the “A” line or the plasticity index is less than 4, and presence of organic matter does not influence the liquid limit as determined in 11.3.2.

11.2.1 Classify the soil as a silt, ML, if the liquid limit is less than 50. See area identified as ML on Fig. 4.

11.2.2 Classify the soil as an elastic silt, MH, if the liquid limit is 50 or greater. See area identified as MH on Fig. 4.

11.3 The soil is an organic silt or clay if organic matter is present in sufficient amounts to influence the liquid limit as determined in 11.3.2.

11.3.1 If the soil has a dark color and an organic odor when moist and warm, a second liquid limit test shall be performed on a test specimen which has been oven dried at 110 ± 5°C to a constant weight, typically over night.

11.3.2 The soil is an organic silt or organic clay if the liquid limit after oven drying is less than 75 % of the liquid limit of the original specimen determined before oven drying (see Procedure B of Practice D 2217).

11.3.3 Classify the soil as an organic silt or organic clay, OL, if the liquid limit (not oven dried) is less than 50. Classify the soil as an organic silt, OL, if the plasticity index is less than 4, or the position of the plasticity index versus liquid limit plot falls below the “A” line. Classify the soil as an organic clay, OL, if the plasticity index is 4 or greater and the position of the plasticity index versus liquid limit plot falls on or above the “A” line. See area identified as OL (or CL-ML) on Fig. 4.

11.3.4 Classify the soil as an organic clay or organic silt, OH, if the liquid limit (not oven dried) is 50 or greater. Classify the soil as an organic silt, OH, if the position of the plasticity index versus liquid limit plot falls below the “A” line. Classify the soil as an organic clay, OH, if the position of the plasticity index versus liquid limit plot falls on or above the “A” line. See area identified as OH on Fig. 4.
11. If less than 30% but 15% or more of the test specimen
is retained on the No. 200 (75-µm) sieve, the words “with
sand” or “with gravel” (whichever is predominant) shall be
added to the group name. For example, lean clay with sand,
CL; silt with gravel, ML. If the percent of sand is equal to the
percent of gravel, use “with sand.”

11.5 If 30% or more of the test specimen is retained on the
No. 200 (75-µm) sieve, the words “sandy” or “gravelly” shall
be added to the group name. Add the word “sandy” if 30% or
more of the test specimen is retained on the No. 200 (75-µm)
sieve and the coarse-grained portion is predominantly sand.
Add the word “gravelly” if 30% or more of the test specimen
is retained on the No. 200 (75-µm) sieve and the coarse-grained
portion is predominantly gravel. For example, sandy lean clay,
CL; gravelly fat clay, CH; sandy silt, ML. If the percent of sand
is equal to the percent of gravel, use “sandy.”

12. Procedure for Classification of Coarse-Grained Soils
(more than 50% retained on the No. 200 (75-µm) sieve)

12.1 Class the soil as gravel if more than 50% of the coarse
fraction [plus No. 200 (75-µm) sieve] is retained on the No. 4
(4.75-mm) sieve.

12.2 Class the soil as sand if 50% or more of the coarse
fraction [plus No. 200 (75-µm) sieve] passes the No. 4
(4.75-mm) sieve.

12.3 If 12% or less of the test specimen passes the No. 200
(75-µm) sieve, plot the cumulative particle-size distribution,
Fig. 5, and compute the coefficient of uniformity, Cu, and
coefficient of curvature, Cc, as given in Eqs 1 and 2.

\[
Cu = \frac{D_{60}}{D_{10}} \tag{1}
\]

\[
Cc = \frac{(D_{50})^2}{(D_{10} \times D_{60})} \tag{2}
\]

where:

- \(D_{10}, D_{50}, \) and \(D_{60}\) = the particle-size diameters corresponding
to 10, 50, and 60%, respectively, passing on the cumulative
particle-size distribution curve, Fig. 5.

Note 9—It may be necessary to extrapolate the curve to obtain the \(D_{10}\)
diameter.

12.3.1 If less than 5% of the test specimen passes the No.
200 (75-µm) sieve, classify the soil as a well-graded gravel,
GW, or well-graded sand, SW, if Cu is greater than or equal to
4.0 for gravel or greater than 6.0 for sand, and Cc is at least 1.0
but not more than 3.0.

12.3.2 If less than 5% of the test specimen passes the No.
200 (75-µm) sieve, classify the soil as poorly graded gravel,
GP, or poorly graded sand, SP, if either the Cu or the Cc
criteria for well-graded soils are not satisfied.

12.4 If more than 12% of the test specimen passes the No.
200 (75-µm) sieve, the soil shall be considered a coarse-
grained soil with fines. The fines are determined to be either
clayey or silty based on the plasticity index versus liquid limit
plot on Fig. 4. (See 9.8.2.1 if insufficient material available for
testing) (see Note 7).

12.4.1 Classify the soil as a clayey gravel, GC, or clayey sand,
SC, if the fines are clayey, that is, the position of the
plasticity index versus liquid limit plot, Fig. 4, falls on or above
the “A” line and the plasticity index is greater than 7.

12.4.2 Classify the soil as a silty gravel, GM, or silty sand,
SM, if the fines are silty, that is, the position of the plasticity
index versus liquid limit plot, Fig. 4, falls below the “A” line
or the plasticity index is less than 4.

12.4.3 If the fines plot as a silty clay, CL-ML, classify the
soil as a silty, clayey gravel, GC-GM, if it is a gravel or a silty,
clayey sand, SC-SM, if it is a sand.

12.5 If 5 to 12% of the test specimen passes the No. 200
(75-µm) sieve, give the soil a dual classification using two
group symbols.

12.5.1 The first group symbol shall correspond to that for a
gravel or sand having less than 5% fines (GW, GP, SW, SP),
and the second symbol shall correspond to a gravel or sand
having more than 12% fines (GC, GM, SC, SM).

12.5.2 The group name shall correspond to the first
group symbol plus “with clay” or “with silt” to indicate the plasticity
characteristics of the fines. For example, well-graded gravel
with clay, GW-GC; poorly graded sand with silt, SP-SM (See
9.8.2.1 if insufficient material available for testing).

Note 10—If the fines plot as a silty clay, CL-ML, the second
group symbol should be either GC or SC. For example, a poorly graded sand
with 10% fines, a liquid limit of 20, and a plasticity index of 6 would be
classified as a poorly graded sand with silty clay, SP-SC.

12.6 If the specimen is predominantly sand or gravel but
contains 15% or more of the other coarse-grained constituent,
the words “with gravel” or “with sand” shall be added to the
group name. For example, poorly graded gravel with sand,
clayey sand with gravel.

12.7 If the field sample contained any cobbles or boulders or
both, the words “with cobbles,” or “with cobbles and boulders”
shall be added to the group name. For example, silty gravel
with cobbles, GM.

13. Report

13.1 The report should include the group name, group
symbol, and the results of the laboratory tests. The particle-size
distribution shall be given in terms of percent of gravel, sand,
and fines. The plot of the cumulative particle-size distribution
curve shall be reported if used in classifying the soil. Report
appropriate descriptive information according to the proce-
dures in Practice D 2488. A local or commercial name or
geologic interpretation for the material may be added at the end
of the descriptive information if identified as such. The test
procedures used shall be referenced.

Note 11—Example: Clayey Gravel with Sand and Cobbles (GC)—
46% fine to coarse, hard, subrounded gravel; 50% fine to coarse, hard,
subrounded sand; 24% clayey fines, LL = 38, PI = 19; weak reaction with
HCl; original field sample had 4% hard, subrounded cobbles; maximum
dimension 150 mm.

In-Place Conditions—firm, homogeneous, dry, brown,
Geologic Interpretation—alluvial fan.

Note 12—Other examples of soil descriptions are given in Appendix
X1.

14. Precision and Bias

14.1 Criteria for acceptability depends on the precision and
bias of Test Methods D 422, D 1140 and D 4318.
**15. Keywords**

15.1 Atterberg limits; classification; clay; gradation; gravel; laboratory classification; organic soils; sand; silt; soil classification; soil tests

**APPENDIXES**

(Nonmandatory Information)

**X1. EXAMPLES OF DESCRIPTIONS USING SOIL CLASSIFICATION**

X1.1 The following examples show how the information required in 13.1 can be reported. The appropriate descriptive information from Practice D 2488 is included for illustrative purposes. The additional descriptive terms that would accompany the soil classification should be based on the intended use of the classification and the individual circumstances.

X1.1.1 **Well-Graded Gravel with Sand (GW)**—73 % fine to coarse, hard, subangular gravel; 23 % fine to coarse, hard, subangular sand; 4 % fines; Cc = 2.7, Cu = 12.4.

X1.1.2 **Silty Sand with Gravel (SM)**—61 % predominantly fine sand; 23 % silty fines, LL = 33, PI = 6; 16 % fine, hard, subrounded gravel; no reaction with HCl; (field sample smaller than recommended). In-Place Conditions—Firm, stratified and contains lenses of silt 1 to 2 in. thick, moist, brown to gray; in-place density = 106 lb/ft³ and in-place moisture = 9 %.

X1.1.3 **Organic Clay (OL)**—100 % fines, LL (not dried) = 32, LL (oven dried) = 21, PI (not dried) = 10; wet, dark brown, organic odor, weak reaction with HCl.

X1.1.4 **Silty Sand with Organic Fines (SM)**—74 % fine to coarse, hard, subangular reddish sand; 26 % organic and silty dark-brown fines, LL (not dried) = 37, LL (oven dried) = 26, PI (not dried) = 6; wet, weak reaction with HCl.

X1.1.5 **Poorly Graded Gravel with Silt, Sand, Cobbles and Boulders (GP-GM)**—78 % fine to coarse, hard, subrounded to subangular gravel; 16 % fine to coarse, hard, subrounded to subangular sand; 6 % silty (estimated) fines; moist, brown; no reaction with HCl; original field sample had 7 % hard, subrounded cobbles and 2 % hard, subrounded boulders with a maximum dimension of 18 in.

**X2. USING SOIL CLASSIFICATION AS A DESCRIPTIVE SYSTEM FOR SHALE, CLAYSTONE, SHELLS, SLAG, CRUSHED ROCK, ETC.**

X2.1 The group names and symbols used in this standard may be used as a descriptive system applied to materials that exist in situ as shale, claystone, sandstone, siltstone, mudstone, etc., but convert to soils after field or laboratory processing (crushing, slaking, etc.).

X2.2 Materials such as shells, crushed rock, slag, etc., should be identified as such. However, the procedures used in this standard for describing the particle size and plasticity characteristics may be used in the description of the material. If desired, a classification in accordance with this standard may be assigned to aid in describing the material.

X2.3 If a classification is used, the group symbol(s) and group names should be placed in quotation marks or noted with some type of distinguishing symbol. See examples.

X2.4 Examples of how soil classifications could be incorporated into a description system for materials that are not naturally occurring soils are as follows:

X2.4.1 **Shale Chunks**—Retrieved as 2- to 4-in. pieces of shale from power auger hole, dry, brown, no reaction with HCl.

After laboratory processing by slaking in water for 24 h, material classified as “Sandy Lean Clay (CL)”—61 % clayey fines, LL = 37, PI = 16; 33 % fine to medium sand; 6 % gravel-size pieces of shale.

X2.4.2 **Crushed Sandstone**—Product of commercial crushing operation; “Poorly Graded Sand with Silt (SP-SM)”—91 % fine to medium sand; 9 % silty (estimated) fines; dry, reddish-brown, strong reaction with HCl.

X2.4.3 **Broken Shells**—65 % gravel-size broken shells; 31 % sand and sand-size shell pieces; 4 % fines; Cc = 2.4, Cu = 1.9; would be classified as “Poorly Graded Gravel with Sand (GP)”.

X2.4.4 **Crushed Rock**—Processed gravel and cobbles from Pit No. 7; “Poorly Graded Gravel (GP)”—89 % fine, hard, angular gravel-size particles; 11 % coarse, hard, angular sand-size particles, dry, tan; no reaction with HCl; Cc = 2.4, Cu = 0.9.
X3. PREPARATION AND TESTING FOR CLASSIFICATION PURPOSES BY THE WET METHOD

X3.1 This appendix describes the steps in preparing a soil sample for testing for purposes of soil classification using a wet-preparation procedure.

X3.2 Samples prepared in accordance with this procedure should contain as much of their natural water content as possible and every effort should be made during obtaining, preparing, and transporting the samples to maintain the natural moisture.

X3.3 The procedures to be followed in this standard assume that the field sample contains fines, sand, gravel, and plus 3-in. (75-mm) material and the cumulative particle-size distribution plus the liquid limit and plasticity index values are required (see 9.8). Some of the following steps may be omitted when they are not applicable to the soil being tested.

X3.4 If the soil contains plus No. 200 (75-µm) particles that would degrade during dry sieving, use a test procedure for determining the particle-size characteristics that prevents this degradation.

X3.5 Since this classification system is limited to the portion of a sample passing the 3-in. (75-mm) sieve, the plus 3-in. (75-mm) material shall be removed prior to the determination of the particle-size characteristics and the liquid limit and plasticity index.

X3.6 The portion of the field sample finer than the 3-in. (75-mm) sieve shall be obtained as follows:

X3.6.1 Separate the field sample into two fractions on a 3-in. (75-mm) sieve, being careful to maintain the natural water content in the minus 3-in. (75-mm) fraction. Any particles adhering to the plus 3-in. (75-mm) particles shall be brushed or wiped off and placed in the fraction passing the 3-in. (75-mm) sieve.

X3.6.2 Determine the air-dry or oven-dry weight of the fraction retained on the 3-in. (75-mm) sieve. Determine the total (wet) weight of the fraction passing the 3-in. (75-mm) sieve.

X3.6.3 Thoroughly mix the fraction passing the 3-in. (75-mm) sieve. Determine the water content, in accordance with Test Method D 2216, of a representative specimen with a minimum dry weight as required in 7.2. Save the water-content specimen for determination of the particle-size analysis in accordance with X3.8.

X3.6.4 Compute the dry weight of the fraction passing the 3-in. (75-mm) sieve based on the water content and total (wet) weight. Compute the total dry weight of the sample and calculate the percentage of material retained on the 3-in. (75-mm) sieve.

X3.7 Determine the liquid limit and plasticity index as follows:

X3.7.1 If the soil disaggregates readily, mix on a clean, hard surface and select a representative sample by quartering in accordance with Practice C 702.

X3.7.1.1 If the soil contains coarse-grained particles coated with and bound together by tough clayey material, take extreme care in obtaining a representative portion of the No. 40 (425-µm) fraction. Typically, a larger portion than normal has to be selected, such as the minimum weights required in 7.2.

X3.7.1.2 To obtain a representative specimen of a basically cohesive soil, it may be advantageous to pass the soil through a ¼-in. (19-mm) sieve or other convenient size so the material can be more easily mixed and then quartered or split to obtain the representative specimen.

X3.7.2 Process the representative specimen in accordance with Procedure B of Practice D 2217.

X3.7.3 Perform the liquid-limit test in accordance with Test Method D 4318, except the soil shall not be air dried prior to the test.

X3.7.4 Perform the plastic-limit test in accordance with Test Method D 4318, except the soil shall not be air dried prior to the test, and calculate the plasticity index.

X3.8 Determine the particle-size distribution as follows:

X3.8.1 If the water content of the fraction passing the 3-in. (75-mm) sieve was required (X3.6.3), use the water-content specimen for determining the particle-size distribution. Otherwise, select a representative specimen in accordance with Practice C 702 with a minimum dry weight as required in 7.2.

X3.8.2 If the cumulative particle-size distribution including a hydrometer analysis is required, determine the particle-size distribution in accordance with Test Method D 422. See 9.7 for the set of required sieves.

X3.8.3 If the cumulative particle-size distribution without a hydrometer analysis is required, determine the particle-size distribution in accordance with Test Method C 136. See 9.7 for the set of required sieves. The specimen should be soaked until all clayey aggregations have softened and then washed in accordance with Test Method C 117 prior to performing the particle-size distribution.

X3.8.4 If the cumulative particle-size distribution is not required, determine the percent fines, percent sand, and percent gravel in the specimen in accordance with Test Method C 117, being sure to soak the specimen long enough to soften all clayey aggregations, followed by Test Method C 136 using a nest of sieves which shall include a No. 4 (4.75-mm) sieve and a No. 200 (75-µm) sieve.

X3.8.5 Calculate the percent fines, percent sand, and percent gravel in the minus 3-in. (75-mm) fraction for classification purposes.
X4. AIR-DRIED METHOD OF PREPARATION OF SOILS FOR TESTING FOR CLASSIFICATION PURPOSES

X4.1 This appendix describes the steps in preparing a soil sample for testing for purposes of soil classification when air-drying the soil before testing is specified or desired or when the natural moisture content is near that of an air-dried state.

X4.2 If the soil contains organic matter or mineral colloids that are irreversibly affected by air drying, the wet-preparation method as described in Appendix X3 should be used.

X4.3 Since this classification system is limited to the portion of a sample passing the 3-in. (75-mm) sieve, the plus 3-in. (75-mm) material shall be removed prior to the determination of the particle-size characteristics and the liquid limit and plasticity index.

X4.4 The portion of the field sample finer than the 3-in. (75-mm) sieve shall be obtained as follows:

X4.4.1 Air dry and weigh the field sample.
X4.4.2 Separate the field sample into two fractions on a 3-in. (75-mm) sieve.
X4.4.3 Weigh the two fractions and compute the percentage of the plus 3-in. (75-mm) material in the field sample.

X4.5 Determine the particle-size distribution and liquid limit and plasticity index as follows (see 9.8 for when these tests are required):

X4.5.1 Thoroughly mix the fraction passing the 3-in. (75-mm) sieve.
X4.5.2 If the cumulative particle-size distribution including a hydrometer analysis is required, determine the particle-size distribution in accordance with Test Method D 422. See 9.7 for the set of sieves that is required.
X4.5.3 If the cumulative particle-size distribution without a hydrometer analysis is required, determine the particle-size distribution in accordance with Test Method D 1140 followed by Test Method C 136. See 9.7 for the set of sieves that is required.
X4.5.4 If the cumulative particle-size distribution is not required, determine the percent fines, percent sand, and percent gravel in the specimen in accordance with Test Method D 1140 followed by Test Method C 136 using a nest of sieves which shall include a No. 4 (4.75-mm) sieve and a No. 200 (75-µm) sieve.
X4.5.5 If required, determine the liquid limit and the plasticity index of the test specimen in accordance with Test Method D 4318.

X5. ABBREVIATED SOIL CLASSIFICATION SYMBOLS

X5.1 In some cases, because of lack of space, an abbreviated system may be useful to indicate the soil classification symbol and name. Examples of such cases would be graphical logs, databases, tables, etc.

X5.2 This abbreviated system is not a substitute for the full name and descriptive information but can be used in supplementary presentations when the complete description is referenced.

X5.3 The abbreviated system should consist of the soil classification symbol based on this standard with appropriate lower case letter prefixes and suffixes as:

<table>
<thead>
<tr>
<th>Prefix</th>
<th>Suffix</th>
</tr>
</thead>
<tbody>
<tr>
<td>s = sandy</td>
<td>s = with sand</td>
</tr>
<tr>
<td>g = gravelly</td>
<td>g = with gravel</td>
</tr>
<tr>
<td>c = cobbles</td>
<td>c = with cobbles</td>
</tr>
<tr>
<td>b = boulders</td>
<td>b = with boulders</td>
</tr>
</tbody>
</table>

X5.4 The soil classification symbol is to be enclosed in parentheses. Some examples would be:

<table>
<thead>
<tr>
<th>Group Symbol and Full Name</th>
<th>Abbreviated</th>
</tr>
</thead>
<tbody>
<tr>
<td>CL, Sandy lean clay</td>
<td>s(CL)</td>
</tr>
<tr>
<td>SP-Sm, Poorly graded sand with silt and gravel</td>
<td>(SP-SM)sg</td>
</tr>
<tr>
<td>GP, poorly graded gravel with sand, cobbles, and boulders</td>
<td>(GP)scb</td>
</tr>
<tr>
<td>ML, gravelly silt with sand and cobbles</td>
<td>g(ML)sc</td>
</tr>
</tbody>
</table>
SUMMARY OF CHANGES

In accordance with Committee D18 policy, this section identifies the location of changes to this standard since the last edition (2000) that may impact the use of this standard.

(1) Added Test Method D 6913 to Section 2 and 7.2.
(2) Corrected 7.2 in selecting size of a sample.
(3) Corrected Example X2.4.3.

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